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## Bharatiya Vidya Bhavan's SARDAR PATEL COLLEGE OF ENGINEERING (An Autonomous Institution Affilliated to University of Mumbai)

DESIGN OF PRESTRESSED CONCRETE

19/12/14.

Total Marks: 100

M.E.(Civil) Sem II (Gray) December 2014

Duration 4hrs MASTER FILE.

NOTE: 1) Question No 1 is compulsory. Answer any 4 from the remaining

2) Use of IS1343-1980, IRC-6, 18, 20 is permitted.

3) Assume the data wherever required and state it clearly

- Q 1 A post tensioned prestressed concrete **bridge girder** of type 1 is shown in the figure.
  - 1) Span of the girder is 22m.

 $_{2}$ )  $f_{p}=1860MPa$ .

3) f<sub>ck</sub>=45MPa. f<sub>ci</sub>=35MPa.

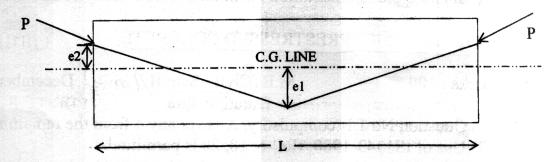
- 4)4 cables 7K-15 i.e. 7strands of 15.2mm diameter From IS-6006-1983, area of single strand is 140mm<sup>2</sup>. f<sub>pe</sub>=.85f<sub>p</sub>
- 5) DL. Bending moment=3000kNm and Shear force=420kN
- 6) LL. Bending moment=1000 kNm and Shear force=140 kN Check the ultimate flexural strength, ultimate shear strength (cracked section) and If  $y_{po}=112.5 \text{mm}$  and  $y_{o}=450 \text{mm}$  design the end block. Stress at centroidal axis due to prestress is 5 MPa.

Top Flange 1200x200mm
Bottom flange 500x400mm
Web 200x800mm

4 Cables 7ply-15.2mm strands in 65mm duct with clear spacing of 50mm Centre of the duct is 100mm from the soffit

Design of Prestressed concrete 19/12/14.

Q 2 a Derive the equation for deflection due to Prestress in a beam with the sloping cable 5 profile as shown in the figure



b A prestressed concrete beam 150mm wide and 450mm deep and 6m span support a load of 5kN/m(UDL) over the entire span. The prestress is 225kN through the area of steel of 300mm<sup>2</sup>. Calculate the deflection of the beam at following stages:

1)working load 2)Cracking load 3)1.5times the cracking load

E<sub>c</sub>=35GPa. m=6 modulus of rupture for concrete =5MPa.

The prestressing cable is parabolic with concentric at supports and dip of 150mm at centre.

## Q3 a Explain the concordant cable

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- b A two span continuous prestressed concrete beam ABC (AB=12m & BC=10m) has a rectangular section (350x750mmdeep). The beam is prestressed by a cable carrying an effective force of 800kN. The cable has a linear profile in span AB and parabolic profile in span BC, The eccentricities of the cable at A is 50mm above the centroidal axis,100mm below centroidal axis at 7m from A,200mm above the centroidal axis at support B and 200mm below centroidal axis at mid span of BC.
  - a) Evaluate the resultant moment developed at B due to prestressing force. and
  - b) Sketch the line of thrust in the beam if it supports a UDL of 10kN/m.
  - c) draw the resultant stress distribution at B for the condition (b)
- Q 4 A post tensioned T-section has 800mm wide and 250mm deep flange and the web is 200mmwide. The area of tensile steel 4000mm<sup>2</sup> is placed at an effective depth of 12 00mm.

  f<sub>p</sub>=1500MPa. f<sub>ck</sub>=45MPa. f<sub>e</sub>=900MPa. after losses. L/d=10.

  Evaluate the ultimate moment capacity of the section if a)Bonded b)Unbonded 3) it is pretensioned beam.
- Q 5 a List five technical advantages of using precast prestressed unit over cast in situ unit.
  - b Explain propped and unpropped construction

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	M.E. Civil- Sem II (Structural), A.T. K.T.	
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	Design of prestressed concrete 19/12/14.	
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)6 a	A post tensioned I-section the top flange is 1000x250mm, bottom flange is 500x350mm, web is 150x800mm. The effective span of 25m and other data is as follows:	10
	a) Superimposed Dead load of 5kN/m.	
	b) Superimposed Live load of 12kN/m.	
	c) $f_{ck}=50MPa.$ , $f_p=1600MPa.$ ,	
	d) Loss in prestress is 20%.	
	e) The permissible tensile stress at transfer is 1.2MPa and that at service is zero. Determine the limits of prestressing force and eccentricities at mid span. Select a prestressing force and eccentricity and calculate the area of steel.	
ь	A prestressed concrete Tee beam has:	10
	flange 1000 x200mm deep and web is 200 x1000mm deep.	
	At a particular section the section is subjected to an ultimate moment of 1800kNm and ultimate shear of 225kN. Calculate the flexure -shear resistance and design the suitable shear reinforcement at a section:	
	i) Effective depth =1100mm ii) f <sub>ck</sub> =50Mpa iii)f <sub>p</sub> =1500MPa.	
	iv) Effective tensile stress at the extreme tensile face of the beam is 19.3MPa, v)Area of prestressing steel=2310mm <sup>2</sup> .	
	vi)Effective prestress in tendons after losses is 900MPa.	

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